## **REPORT ON**

STATIC AXIAL COMPRESSIVE PILE LOAD TEST ON PHC PILE

**TEST PILE ID: P-91** 

**OWNER** 

## SHOPNOSHUR RESIDENT'S SOCIETY

**CONSTRUCTION COMPANY** 

SAMUDA CONSTRUCTION LTD.

PROJECT: PROPOSED (20) TWENTY STORIED RESIDENTIAL BUILDING WITH 02 BASEMENTS SHOPNOSHUR RESIDENTS, PLOT-2589-2590, ROAD- 22, BLOCK- M, BASHUNDHARA R/A, DHAKA.

JANUARY – 2024

## **TEST & REPORT PREPARED BY**

## **BD SOIL & FOUNDATION**

Corporate Office:

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Pressure Calculation & Pile Load Test Sequence

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Static Pile Load Test Records (Field Data Computer Sheet)

Field Data Records

Photographs

#### 1.0 INTRODUCTION

This report presents the test results of monotonic Static Axial Pile Load tests on PHC Pile for the site of **Proposed (20) Twenty Storied Residential Building with 02 Basements ShopnoShur Residents, Plot-2589-2590, Road- 22, Block- M, Bashundhara R/A, Dhaka..** The Contractor for the project Samuda Construction Ltd. installed 500mm diameter having of 23.0m length PHC test pile for the foundation of the structure. Pile designated as P-91 was tested under static axial compressive load to confirm the load capacity of the pile. Pile load testing work was awarded to *"BD Soil & Foundation"* a Geotechnical Engineering firm Located in Dhaka. This tests were carried out with a view to confirm the carrying capacity of single pile under monotonic static load and to know the settlement behavior of this piles under test. This test was carried out on 18<sup>th</sup> to 19<sup>th</sup> January 2024

#### 2.0 OBJECTIVE

Pile load test have been carried out to achieve following objectives-

- To determine the settlement under working load
- To confirm the adequacy of design bearing capacity
- As proof of acceptability
- Determine allowable bearing capacity

Maintained load in axial compression test was performed according to ASTM D-1143 to accomplish the objectives

#### 3.0 SCOPE OF WORK

- Scope of work were set as follows -
- Materials and instruments required for the test
- Placement of hydraulic driving machine as loading platform
- Placement of concrete block on platform
- Installation of reference beam
- Finishing of pile top for the test of hydraulic jack, Installation of pile collar, placement of jack & dial gauge reading etc.
- Ensure safety provisions

Recording of time, load & settlement data during loading and unloading.

Submission of report that will include following-

- Preparation of related graph
- Analysis of recorded data
- Interpretation of results

#### 4.0 TEST METHODOLOGY

The general guide line of ASTM 1143 "Standard Method of Testing Piles under Static Axial Compressive Load" was followed.

Static Axial Compressive Load was provided by hydraulic jack acting against hydraulic driving machine platform counter weight Kent ledge as reaction system. Kent ledge was prepared, by hydraulic driving machine with concrete block.

Following equipment were used in the testing scheme:

a.	Hydraulic jack	01 no (Pre-calibrated combined with pressure gauge)
b.	Pressure gauge	01 no
c.	Dial gauge	02 nos (Pre-calibrated)
d.	Hydraulic pump	01 no

The load on pile was applied by using hydraulic jack seated on bearing plate. Piston pump was used pressure. Calibrated hydraulic jack & pressure gauge was used to monitored load.

10% of proof load was added with counter weight to avoid the tilting of Kentledge. Total load of the Kent ledge was about 500.00 Ton.

Two settlement gauge of accuracy up 0.01 mm and stroke of 50 mm were used to measure settlement with respect to a horizontal reference beam fixed on the ground.

#### 5.0 PHYSICAL DESCRIPTION OF TEST PILE AND EQUIPMENT

Physical description of test pile and equipment use are provided in Table-1 & Table-2 bellow

Test pile	Diameter	Pile Head	Embedment Length	PC Bar	Spiral	Concrete	Applied
no	(mm)	Туре	(m)		wire dia	Grade	Load (kg)
P – 91	500	Cut off head	23.0	10 nos (9mm)	4.3mm	M-70	320,000

Table-1Physical description of test pile (PHC Pile)

Page-2

Test pile	D D:	Jack calibration	n combined wit	h pressure gauge	Dial ga	uge
no	Ram Dia	Pressure gauge Range	Calibration Date	Regression equation	Sensitivity	Range
P – 91	35.00 cm	0-1000 Kg/cm <sup>2</sup>	12.11.23	Y=9.2211X - 210.58	0.01 mm	0-50mm

Table-2 Description of Equipment used

#### 6.0 INTERPRETATION OF TEST RESULT

The load-Settlement–time curves and load-Settlement curve for the test pile is shown respectively. A number of arbitrary or empirical methods are used to serve as criteria for determining the allowable and ultimate load Carrying Capacity from pile load test. Some are based on maximum permissible gross or net Settlement as measured at the pile butt while the others are based on the performance of the pile during the progress of testing (Chellis, 1961; Whitaker, 1976; Poulos and Davis, 1980; Fuller, 1983). Most commonly practiced criteria used for evaluating the ultimate and allowable load Carrying Capacity of pile in Bangladesh are given below.

#### **Definition of Failure Load**

The maintained load (ML) test is commonly used to determine the ultimate bearing capacity of the pile. Whenever a well-defined load has not been obtained, following definitions of failure are adopted:

# Source: A Short Course in Foundation Engineering, 2<sup>nd</sup> edition (by <u>Noel Simons</u> & <u>Bruce</u> <u>Menzies</u>).

- I. The failure load is that which causes settlement equal to 10% of the the pile diameter, making allowance for the elastic shortening of the pile itself that may be significant for long piles
- II. The failure load is that at which the rate of settlement continues undiminished without further increment of load, unless of course the rate is so slow as to indicate that it is due to consolidation of soil
- III. The failure load is the load where the load settlement curve has its minimum radius of curvature.

IV. Drawing tangents to the initial and final points of the load settlement curve and taking the point of intersection as the failure load.

#### **Definition of Ultimate Load/ Allowed Load:**

#### According to BNBC (2020, Part 6, Chapter -3, Article -3.10.1.17, Page : 6-181):

#### (II) Safe Load for Single Pile :

(a). Two thirds of the final load at which the load displacement attains a value of 12 mm unless otherwise required in a given case on the basis of nature and type of structure in which case, the safe load should be corresponding to the stated total displacement permissible.

- (b). Fifty (50) percent of the final load at which the total displacement equals to 10 percent of piles diameter case of uniform diameter piles and 7.5 percent of bulb diameter in case of under-reamed piles.
- (III) According to Indian Standard Code of practice (IS: 2911 1985), allowable pile capacity is smaller of the following

(a) The safe load is Two-thirds of the final load at which the total displacement attains a value of 12 mm unless otherwise required in a given case on the basis of nature and type of structure in which case, the safe load should be corresponding to the stated total displacement permissible.

(b). 50 percent of the final load at which the total displacement equal 10 percent of the pile diameter in case of uniform diameter piles and 7.5 percent of bulb diameter in case of under-reamed piles.

#### **Davisson Rule**

I. This method was proposed by Davisson (1973) as the load corresponding to movement that exceeds the elastic compression of the pile (PL/AE) by a value of 0.15inch plus the diameter divided by 120. Where d is the pile diameter in inches. The slope of elastic shortening line is approximately 20 degree or drawn to initial straight portion of load-settlement curve. Davisson line though provides most conservative results especially in the case of bored piles where the result becomes impractically conservative.

#### 7.0 OBSERVATION

The result of the test is shown in the figures and tables enclosed, summary of the test result are as follows:

Test result is provided in Table-3 bellow; also load test curves of Appendix are referred.

Test The and Test Results, Ref. Load Settlement Curves & Field Data					
Test pile		Die of		Test Result	
no	length of Pile	Pile	Max <sup>m</sup> applied load	Gross Settlement	Net Settlement
P-91	23.0m	500mm	320,000kg	17.750mm	4.955mm

 Table-3

 Test Pile and Test Results, Ref. Load Settlement Curves & Field Data

Page-4

Ultimate & allowable load capacities by different methods are presented in Table-4 & 5

Ultimate Load Capacity of the Test Pile					
Test Pile No.	Max <sup>m</sup> applied	Gross	Ultimate Load Capacity in kg		
	load	(mm)	BNBC 2020 & IS: 2911	Davisson (1973)	
P-91	320,000kg	17.750mm	280,000kg	>320,000	

Table-4Ultimate Load Capacity of the Test Pile

Table-5Allowable Load Capacity of the Test Pile

Test Pile	Max <sup>m</sup> applied	Gross Settlement	Allowable Load Capacity in kg		
No.	load	(mm)	BNBC 2020 & IS: 2911	Davisson (F.S=2.0)	
P-91	320,000kg	17.750mm	186,666kg	>160,000kg	

#### 8.0 CONCLUSION:

- Thus, we can say that the allowable load carrying capacity of the piles have been fulfilled of BNBC, IS & Davisson under static axial compressive pile load test
- Present Situation the allowable load of the tested pile P-91 is greater than the design load of 160,000kg according to BNBC, IS: 2911 & Davisson.
- So design engineer specific criteria for this particular project will supersede the allowable & safe capacity of the tested pile
- > Produced load settlement curve excerpts very consistent shape as for a regular pile.

#### 9.0 CLOSURE

We appreciate the opportunity to perform this test for you and have pleasure in submitting this report. Please contact us when we can be further service to you.

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Engr. Jahurul Islam CEO & Geotechnical Engineer Bd Soil & Foundation MIEB No. M-40447

### APPENDIX A1

LOAD TEST CURVES (Ultimate Load as Per BNBC, IS, Davisson Rule)

### PRESSURE CALCULATION & LOADING SEQUENCE

### **CALIBRATION CHART**

### FIELD DATA RECORD COMPUTER SHEET

### **RAW FIELD DATA RECORDS**

**PHOTOGRAPHS** 

#### LOAD TEST PLOTS





#### LOAD TEST PLOTS

Project: Proposed (20) Twenty Storied Residential Building with 02 Basements ShopnoShur Residents, Plot-2589-2590, Road- 22, Block- M, Bashundhara R/A, Dhaka.

		Pile ID: P-91	
Design Load (Kg):	160,000	Maximum Settlement:	17.750mm
Max. App. Load (Kg):	320,000	Net Settlement:	4.955mm



#### Load Vs Settlement Plot

Load (Kg)

### DAVISSON RULES CURVE OF PILE LOAD TEST

# Project: Proposed (20) Twenty Storied Residential Building with 02 Basements ShopnoShur Residents, Plot-2589-2590, Road- 22, Block- M, Bashundhara R/A, Dhaka.

#### Pile ID: P-91

P-91	Pile Concrete Grade:	70.00
500mm	Modulus of Elasticity of Concrete:	4690kg/mm <sup>2</sup>
23.0m	Elastic Shortening for Test Load:	14.86
160,000	Offset Limit Load Settlement:	7.977
320,000	Davisson's Offset Limit Method:	N.A
17.7500mm	Soil Settlement by Test Load:	N.A
4.9550mm	Soil Rebound/	N.A
	P-91 500mm 23.0m 160,000 320,000 17.7500mm 4.9550mm	P-91Pile Concrete Grade:500mmModulus of Elasticity of Concrete:23.0mElastic Shortening for Test Load:160,000Offset Limit Load Settlement:320,000Davisson's Offset Limit Method:17.7500mmSoil Settlement by Test Load:4.9550mmSoil Rebound/

#### Load Vs Settlement & Davisson's Offset Limit Method Curve





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## STRENGTH OF MATERIALS LABORATORY

#### **Calibration of Hydraulic Jack**

BRTC No.:	1103-05146/CE/23-24	Date: 07/11/2023
Reference :	Letter	Date: 07/11/2023
Sent By:	M. R. Foundation	
Project:	Construction of Two Lane RCC Box Girder Flyow	er at New Bilshimla Railway Crossing
Machine:	Hydraulic Jack ID: BD-01 (Capacity: 750 Ton)	Pump ID: MRF-01, Motorized
Jack Information:	Piston Dia: 350 mm; Body Dia: 445 mm; Body H	eight: 438 mm
Pressure Gauge:	Range: 0-1000 kg/cm2 Gauge ID: WIKA; M	IR-01 (Black)
Calibration Device:	Load Column: ELE 1052-10-6096 (3000 kN)	
Date of Calibration:	12.11.2023	



Gauge Reading (kg/cm2)	Actual Jack Load (kN)	Calibrated Jack Load (kN)
0.0	0.0	-210.58
40.0	88.7	158.27
80.0	429.5	527.11
100.0	642.8	711.53
140.0	1033.3	1080.37
180.0	1427.2	1449.22
200.0	1628.0	1633.64
240.0	2014.0	2002.48
280.0	2407.9	2371.32
300.0	2594.1	2555.75
340.0	2938.3	2924.59

Note: 1 Metric-Ton = 9.806 kN

Warning: Calibration is valid only when the above mentioned Jack and Pressure Gauge pair are used together as they are calibrated. Re-calibration shall be needed if any of the above Jack or Pressure Gauge is changed/replaced or repaired.

Countersigned by :

Prof. Dr. Hasib Mohammed Ahsan

Test-in-Charge Department of Civil Engineering BUET, Dhaka-1000, Bangladesh





Professor Department of Civil Engineering BUET, Dhaka-1000, Bangladesh



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### STRENGTH OF MATERIALS LABORATORY

**Calibration of Pressure Gauge** 

BRTC No.:	1103-05146/CE/23-24	Date: 07/11/2023
Reference :	Letter	Date: 07/11/2023
Sent by:	M. R. Foundation	
Project:	Construction of Two Lane RCC Box Girder Flyover at New E	Bilshimla Railway Crossing
Equipment:	Pressure Gauge: WIKA; Gauge ID: MR-01	
Capacity:	0-1000 kg/cm <sup>2</sup>	ANN SAME
Date of Calibration:	09/11/2023	



Note: Pressure Gauge was received in unsealed condition.

$$1 \text{ kg/cm}^2 = 14.223 \text{ psi}$$

Countersigned by :

Prof. Dr. Hasib Mohammed Ahsan Test-in-Charge Department of Civil Engineering BUET, Dhaka-1000, Bangladesh



Observed Pressure (kg/cm <sup>2</sup> )	Actual Pressure (kg/cm²)	±% Error		
60	37.7	59.03%		
100	78.5	27.36%		
160	160.6	-0.38%		
200	186.1	7.47%		
////260	254.4	2.2%		
300	290.6	3.23%		
360	360.0	0.01%		
400	404.3	-1.07%		
460	461.9	-0.42%		
500	506.3	-1.24%		
560	565.9	-1.05%		
600	606.7	-1.11%		
660	668.4	-1.26%		
700	710 7	-1.51%		



Dr. Munaz Ahmed Noor Professor Department of Civil Engineering BUET, Dhaka-1000, Bangladesh

#### BUETCE 0459395



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## GEOTECHNICAL ENGINEERING LABORATORY

Calibration of Dial Gauge (ASTM E4 - 16)

BRTC No.:	1102-85173/22-23/CE dated 4.3.23
Reference :	Letter dated 4.3.23
Sent by:	Sub Assistant Engineer, BD Soil & Foundation, Block-C, Mirpur, Dhaka
Project:	Mohtoshin Ali High School, Kulaura, Moulvibazar
Dial Gauge:	Gauge ID - Mitutoyo; 9MM437 (on side)
Range:	0-50 mm (1 Div=0.01 mm)
Date of Calibratio	m: 5.3.23



Observed Dial Reading (Div)	Actual Reading (mm)	±% Error		
160.8	1.5875	1.26%		
255.3	2.5400	0.49%		
319.0	3.1750	0.47%		
509.0	5.0800	0.2%		
635.5	6.3500	0.08%		
762.3	7.6200	0.03%		
1271.0	12.7000	0.08%		
2540.5	25.4000	0.02%		
3811.5	38.1000	0.04%		
4446.5	44.4500	0.03%		

Countersigned by :



Dr. Hasib Mohammed Ahsan Test-in-Charge Department of Civil Engineering BUET, Dhaka-1000, Bangladesh



A BIRG-1000, Barris

Calibrated by :

5.3.23

Dr. Tahmeed M. Al-Hussaini Professor Department of Civil Engineering BUET, Dhaka-1000, Bangladesh



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### GEOTECHNICAL ENGINEERING LABORATORY

Calibration of Dial Gauge (ASTM E4 – 16)	
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BRTC No .:	1102-85173/22-23/CE dated 4.3.23
Reference :	Letter dated 4.3.23
Sent by:	Sub Assistant Engineer, BD Soil & Foundation, Block-C, Mirpur, Dhaka
Project:	Mohtoshin Ali High School , Kulaura, Moulvibazar
Dial Gauge:	Gauge ID - Mitutoyo; AHU489 (on dial)
Range:	0-50 mm (1 Div=0.01 mm)

Date of Calibration: 5.3.23



Observed Dial Reading (Div)	Actual Reading (mm)	±% Error
159.8	1.5875	0.63%
255.0	2.5400	0.39%
318.3	3.1750	0.24%
508.5	5.0800	0.1%
636.5	6.3500	0.24%
762.8	7.6200	0.1%
1271.8	12.7000	0.14%
2542.0	25.4000	0.08%
3813.5	38.1000	0.09%
4448.5	44.4500	0.08%

Countersigned by :

0

Dr. Hasib Mohammed Ahsan Test-in-Charge Department of Civil Engineering BUET, Dhaka-1000, Bangladesh



Calibrated by :

5.3.23

Dr. Tahmeed M. Al-Hussaini Professor Department of Civil Engineering BUET, Dhaka-1000, Bangladesh

#### Project: Proposed (20) Twenty Storied Residential Building with 02 Basements ShopnoShur Residents, Plot-2589-2590, Road- 22, Block-M, Bashundhara R/A, Dhaka.

Individual Pile Under Static Axial Compressive Load (ASTM-D-1143)

#### Pressure Calculation for Pile: P-91

				Dia of Ram(cm)	)=	35.00				
Des	sing Load (Kg)=	160,000		Area of $\operatorname{Ram}(\operatorname{cm}^2)$ = 962.12						
Maxm	Fest Load (Kg)=	320,000		Regression Equal, Y(kN) = 9.2211X(kg/cm2) - 210.58						
(1	2×design load)			Pressure Gauge	Capacity 1000 H	Kg/cm^2				
				Calibration Date:12.11.2023 from BUET						
		Theoritical				As Planned				
	% of		Observed	Observed	LOAD	% of	Holding	Reading		
LOADING STEPS	160,000	LOAD (Ka)	Pressure	Pressure		160,000	Time	Intervel		
	(Kg)Load	( <b>N</b> g)	$(kg/cm^2)$	$(kg/cm^2)$	( <b>N</b> g)	(Kg)Load	(Minute)	(Minute)		
1 <sup>st</sup> Increment	25	40,000	65.37	65	39,648	24.78	60	15		
2 <sup>nd</sup> Increment	50	80,000	107.91	110	81,964	51.23	60	15		
3 <sup>rd</sup> Increment	75	120,000	150.45	150	119,578	74.74	60	15		
4 <sup>th</sup> Increment	100	160,000	192.99	190	157,192	98.25	60	15		
1 <sup>st</sup> Decrement	75	120,000	150.45	150	119,578	74.74	20	10		
2 <sup>nd</sup> Decrement	50	80,000	107.91	110	81,964	51.23	20	10		
3 <sup>rd</sup> Decrement	25	40,000	65.37	65	39,648	24.78	20	10		
4 <sup>th</sup> Decrement	0	0	0.00	0	0	0.00	60	15		
5 <sup>th</sup> Increment	50	80,000	107.91	110	81,964	51.23	20	10		
6 <sup>th</sup> Increment	100	160,000	192.99	190	157,192	98.25	20	10		
7 <sup>th</sup> Increment	125	200,000	235.52	235	199,508	124.69	60	15		
8 <sup>th</sup> Increment	150	240,000	278.06	280	241,824	151.14	60	15		
9 <sup>th</sup> Increment	175	280,000	320.60	320	279,438	174.65	60	15		
10 <sup>th</sup> Increment	200	320,000	363.13	363	320,000	200.00	С	D		
5 <sup>th</sup> Decrement	150	240,000	278.06	280	241,824	151.14	60	15		
6 <sup>th</sup> Decrement	100	160,000	192.99	190	157,192	98.25	60	15		
7 <sup>th</sup> Decrement	50	80,000	107.91	110	81,964	51.23	60	15		
8 <sup>th</sup> Decrement	0	0	0.00	0	0	0.00	60	15		

Notes

#### **Holding Time :**

A = Any time, if the rate of settlement is less than 0.25 mm/h

B = Max 2 h, if the rate of settlement is greater than 0.25 mm/h

C = Any time after 12 h, if the butt settlement is not greater than 0.25 mm/h in 1 h but otherwise 24 h

#### **Reading Time:**

D = At interval 15 min for for 1st 1 h then, 1 h for next 11 h, then 2h.

Project: Proposed (20) Twenty Storied Residential Building with 02 Basements ShopnoShur Residents, Plot-2589-2590, Road- 22,										
Nama of the	Client:		Blo ShoppoShur	DCK- M, Ba	<u>ishundhara R</u> Society	A, Dhaka.		ACTM D 1142	01	
Construction	Compony	L.		Dia of Part (cm): 25.00			-81			
Test Pile No:	Company:		P-91	nstruction .	Lta	$\Delta rea of Ram (cm2) 962 12$				
Pile Diameter	<b></b>		500mm			Area of Kam (cm ) 902.12				
Length of Pil	e.		23.0m			Max App Lo	ad (Kg).	320,000		
Pile Type:			PHC			Date of driving	2:	16.01.2024		
Pile Details:	T	nickness: 80m	m. PC Bar:	10nos(9mr	n).	Date of Calibra	ation:	12.11.2023		
	S	spiral wire: 4	4.3mm, Cond	crete Grade	: M-70	Regression Eq	ual, $Y_{(kN)} = 9$	.2211X <sub>(kg/cm2)</sub> - 2	10.58	
Maximum Se	ettlement:		17.750mm			Test Starting I	Date:	18-Jan-24		
Net Settlemen	nt:		4.955mm			Test Complet	Date:	19-Jan-24		
Date	Load on	Percent Of	Observed	Elapsed	Cumulative	Dial Gage	Reading	Average		
&	Test Pile	Design	Pressure	Time	Elapsed	M1	M2	Settlement	Remarks	
Time	(Kg)	Load (%)	$(kg/cm^2)$	(min)	Time (min)	(mm)	(mm)	(mm)		
Col-1	Col-2	Col-3	Col-4	Col-5	Col-6	Col-7	Col-8	Col-10	Col-11	
18-Jan-24										
10:50 AM	0.00	0.00	0	0	0	0	0	0.000	Starting	
10:50 AM	39,648	24.78	65	0	0	0.30	0.24	0.270	Load	
11:05 AM	"	"	"	15	15	0.32	0.24	0.280		
11:20 AM	"		"	15	30	0.32	0.24	0.280		
11:35 AM	"		"	15	45	0.32	0.24	0.280		
11:50 AM	"		"	15	60	0.32	0.24	0.280		
11:50 AM	81,964	51.23	110	0	60	2.54	2.72	2.630	Load	
12:05 PM	"	"	"	15	75	2.56	2.76	2.660		
12:20 PM	"	"	"	15	90	2.56	2.77	2.665		
12:35 PM	"		"	15	105	2.57	2.78	2.675		
12:50 PM	"	"	"	15	120	2.57	2.78	2.675		
12:50 PM	119,578	74.74	150	0	120	4.94	4.86	4.900	Load	
1:05 PM	"	"	"	15	135	4.99	4.89	4.940		
1:20 PM	"	"	"	15	150	5.01	4.91	4.960		
1:35 PM	"	"	"	15	165	5.02	4.92	4.970		
1:50 PM	"	"	"	15	180	5.03	4.92	4.975		
1:50 PM	157,192	98.25	190	0	180	6.29	6.21	6.250	Load	
2:05 PM	"	"	"	15	195	6.30	6.23	6.265		
2:20 PM	"	"	"	15	210	6.30	6.24	6.270		
2:35 PM	"	"	"	15	225	6.30	6.24	6.270		
2:50 PM	"	"	"	15	240	6.30	6.24	6.270		
2:50 PM	119,578	74.74	150	0	240	6.30	6.23	6.265	Un-Load	
3:00 PM	"	"	"	10	250	6.28	6.22	6.250		
3:10 PM	"	"	"	10	260	6.27	6.20	6.235		
3:10 PM	81,964	51.23	110	0	260	5.34	5.43	5.385	Un-Load	
3:20 PM	"	"	"	10	270	5.34	5.43	5.385		
3:30 PM	"	"	"	10	280	5.31	5.40	5.355		
3:30 PM	39,648	24.78	65	0	280	3.60	3.71	3.655	Un-Load	
3:40 PM	"	"	"	10	290	3.56	3.71	3.635		
3:50 PM	"	"	"	10	300	3.55	3.70	3.625		
3:50 PM	0	0.00	0	0	300	1.31	1.67	1.490	Un-Load	

Signature Consultant Signature Contractor Signature Client

Date	Load on	Percent Of	Observed	Flansed	Cumulative	Dial Gage	Reading	Average	
&	Test Pile	Design	Pressure	Time	Elapsed	M1	M2	Settlement	Remarks
Time	(Kg)	Load (%)	(kg/cm <sup>2</sup> )	(min)	Time (min)	(mm)	(mm)	(mm)	
Col-1	Col-2	Col-3	Col-4	Col-5	Col-6	Col-7	Col-8	Col-10	Col-11
4:05 PM	"	"	"	15	315	1.25	1.62	1.435	
4:20 PM	"	"	"	15	330	1.22	1.59	1.405	
4:35 PM	"	"	"	15	345	1.19	1.58	1.385	
4:50 PM	"	"	"	15	360	1.17	1.57	1.370	
4:50 PM	81,964	51.23	110	0	360	3.18	3.15	3.165	Load
5:00 PM	"	"	"	10	370	3.20	3.18	3.190	
5:10 PM	"	"	"	10	380	3.21	3.20	3.205	
5:10 PM	157,192	98.25	190	0	380	6.42	6.15	6.285	Load
5:20 PM	"	"	"	10	390	6.45	6.16	6.305	
5:30 PM	"	"	"	10	400	6.46	6.18	6.320	
5:30 PM	199,508	124.69	235	0	400	7.70	7.06	7.380	Load
5:45 PM	"	"	"	15	415	7.72	7.08	7.400	
6:00 PM	"	"	"	15	430	7.73	7.10	7.415	
6:15 PM	"	"	"	15	445	7.74	7.12	7.430	
6:30 PM	"	"	"	15	460	7.74	7.14	7.440	
6:30 PM	241,824	151.14	280	0	460	9.90	9.57	9.735	Load
6:45 PM	"	"	"	15	475	9.95	9.60	9.775	
7:00 PM	"	"	"	15	490	9.99	9.64	9.815	
7:15 PM	"	"	"	15	505	10.04	9.70	9.870	
7:30 PM	"	"	"	15	520	10.07	9.72	9.895	
7:30 PM	279,438	174.65	320	0	520	12.06	11.83	11.945	Load
7:45 PM	"	"	"	15	535	12.10	11.90	12.000	
8:00 PM	"	"	"	15	550	12.14	11.93	12.035	
8:15 PM	"	"	"	15	565	12.17	11.96	12.065	
8:30 PM	"	"	"	15	580	12.18	11.98	12.080	
8:30 PM	320,000	200.00	363	0	580	16.48	17.94	17.210	Load
8:45 PM	"	"	"	15	595	16.55	18.02	17.285	
9:00 PM	"	"	"	15	610	16.61	18.11	17.360	
9:15 PM	"	"	"	15	625	16.70	18.18	17.440	
9:30 PM	"	"	"	15	640	16.76	18.24	17.500	
10:30 PM	"	"	"	60	700	16.80	18.28	17.540	
11:30 PM	"	"	"	60	760	16.85	18.34	17.595	
12:30 AM	"	"	"	60	820	16.91	18.42	17.665	
1:30 AM	"	"	"	60	880	16.96	18.47	17.715	
2:30 AM	"	"	"	60	940	16.98	18.49	17.735	
3:30 AM	"	"	"	60	1,000	17.00	18.50	17.750	
4:30 AM	"	"	"	60	1,060	17.00	18.50	17.750	
5:30 AM	"	"	"	60	1,120	17.00	18.50	17.750	
6:30 AM	"	"	"	60	1,180	17.00	18.50	17.750	
7:30 AM	"	"	"	60	1,240	17.00	18.50	17.750	
8:30 AM	"	"	"	60	1,300	17.00	18.50	17.750	

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Date	Load on	Percent Of	Observed	Observed Elapsed Cumulative Dial Gage Reading		Average			
&	Test Pile	Design	Pressure	Time	Elapsed	M1	M2	Settlement	Remarks
Time	(Kg)	Load (%)	(kg/cm <sup>2</sup> )	(min)	Time (min)	(mm)	(mm)	(mm)	
Col-1	Col-2	Col-3	Col-4	Col-5	Col-6	Col-7	Col-8	Col-10	Col-11
8:30 AM	241,824	151.14	280	0	1,300	16.51	18.26	17.385	Un-Load
8:45 AM	"	"	"	15	1,315	16.47	18.21	17.340	
9:00 AM	"	"	"	15	1,330	16.45	18.19	17.320	
9:15 AM	"	"	"	15	1,345	16.44	18.18	17.310	
9:30 AM	"	"	"	15	1,360	16.44	18.18	17.310	
9:30 AM	157,192	98.25	190	0	1,360	12.57	14.67	13.620	Un-Load
9:45 AM	"	"	"	15	1,375	12.50	14.59	13.545	
10:00 AM	"	"	"	15	1,390	12.50	14.59	13.545	
10:15 AM	"	"	"	15	1,405	12.50	14.59	13.545	
10:30 AM	"	"	"	15	1,420	12.50	14.59	13.545	
10:30 AM	81,964	51.23	110	0	1,420	8.59	9.24	8.915	Un-Load
10:45 AM	"	"	"	15	1,435	8.55	9.24	8.895	
11:00 AM	"	"	"	15	1,450	8.54	9.24	8.890	
11:15 AM	"	"	"	15	1,465	8.52	9.21	8.865	
11:30 AM	"	"	"	15	1,480	8.52	9.21	8.865	
11:30 AM	0	0.00	0	0	1,480	4.95	5.20	5.075	Un-Load
11:45 AM	"	"	"	15	1,495	4.87	5.14	5.005	
12:00 PM	"	"	"	15	1,510	4.81	5.11	4.960	
12:15 PM		"	"	15	1,525	4.80	5.11	4.955	
12:30 PM	"	"	"	15	1,540	4.80	5.11	4.955	

BD SOIL & FOUNDATION

Geotechnical Engineering & Consultant Services

## STATIC AXIAL PILE LOAD TEST FIELD DATA RECORD SHEET

	ST	ATIC AXL	AL PILE J	LOAD I	EST FIEL	D DATA I	ECORD S	HEET Data Recod	Sheet of
Project:	Sha	pmu s	hur				Fleid	Dina Xee	
Name of t	he Client:					Dia of Ra	m (cm):	35.00	
Constructi	on Compa					Area of B	$am(cm^2)$	962.1	2
Test Dile N		- 91				Decion Lo	ad (Kg): 1	60,000	
Pile Diam	NO. F	- 21				May Test	Load (Kg):	320,00	0
I enoth of	Dileer; E	$\frac{1}{2}$				Date of D	riving: 16	-01-20	24
Dile T	Pile:	2311				Date of C	alibration: 19	2-11-2	023
The Type:			HPC Pile			Date of C	tion: U- 9.	2211×-	210.58
Test Meth	od:	4-1	ASTM D	1143-81		Reg. equa	Data: 10	2-01-2	024
Maximum	Settlemen	<u>t: 1X</u>	15 m	m		Test Start	ng Date: 1	3-01-2	024
Net Settle	ment:	4.9	55 mr	n		Test Com	blet Date: L		
Date	Load on	Percent Of	Observed	Elapsed	Cumulative	Dial Gag	e Reading	Average	Bomarks
&	Test Pile	Desion	Pressure	Time	Elapsed	M1	M2	Settlement	Kentarks
Time	(Kg)	Load (%)	$(kg/cm^2)$	(min)	Time (min)	(mm)	(mm)	(mm)	2.1.10
Col-1	Col-2	Col-3	Col-4	Col-5	Col-6	Col-7	Col-8	Col-9	COFIO
10'50	39640	25	65	0	00.0	0.30	0.24	0.27	
11.05	07,010	20	0.2	15		0.32	0.24	0.28	
11.00				15		0.32	0.24	0.28	
11:25				15		0.32	0.24	0.28	
11.55				16		0.32	0.24	0.28	
11.50				15					
MICO	(MO())	E.	110	D		2.54	2.72	2.63	
11.50	81764	50	10	100		2.56	2.76	2.66	
212:05				15		2.56	2.78	2.665	
12:20				15		2.57	2.78	2.675	
12:35				15		2.57	2.78	2.675	
12:50	20570	-15	150			2.51	213		
40.01	119578	72	150	~		11.94	11.86	4.90	
12:50				10	·	4.99	11.89	4.94	
1:05				15		5.11	11.01	4.96	
1:20				15		5.01	4 21	4.97	
1:35				15		5.0.2	4.72	11.975	
1:50				13		3.03	492	4.775	
1150	157192	100	190	0		6.29	6.21	6.25	
2'05	10/02			15		6.30	6.23	6.265	
2:20				15		6.30	6.24	6.27	
2:35				15		6.30	6.24	6.27	
2:50				15		6.30	6.24	6.23	

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	51	ATIC AXIA	AL PILE I	DAD T	EST FIEL	DAIAI	Field 1	Data Recod	Sheet of	2
Date	Loadon	Percent Of	Observed	Flansed	Cumulative	Dial Ga	ge Reading	Average	Γ.	
&	Test Pile	Design	Pressure	Time	Elansed	MI	M2	Settlement	Remarks	
Time	(Kg)	Load (%)	$(kg/cm^2)$	(min)	Time (min)	(mm)	(mm)	(mm)	0.110	-
Col-1	Col-2	Col-3	Col-4	Col-5	Col-6	Col-7	Col-8	Col-9	COFIU	ima "
2:50	11,9578	75	150	0		6.30	6.23	6.265	Unlove	
3:00				10		6.28	6.22	6.25		
3:10				10		6.27	6.20	6.23		
								= 105	1220 100	ling
3:10	81964	50	110	0		5.34	5.43	5.200	UNINU	
3:20				10		5.34	19.43	0,380		
3:30				10		5.31	5.40			
							10 00	0 155	Unloadi	ng
3:30	39648	25	65	D		3.60	3.71	2.600		Ū
3:40	- 10 10			10		3.56	13. XL	3.600		
3:50				10		3.55	3.70			
					1	1.01	17	149	Unloo	ling
3:50	00	00	DD	0	6.1	1.31	1.61	1135		0
4:05				15	Strail 1	1.25	1.59	1.405		
4:20				15		1.22	1.50	1.385		
4:35				15		1.17	1.67	1.37		
4:50				15		1.11	1.91	101	1 1.	
						0218	3.15	3.165	loodi	ng
4:50	81964	50	110	0		300	3.18	3.19		$\Box$
5:00			100	10		3.01	3.20	3.205		
5:10				10		0121	-			_
		-1-0	100			6.42	6.15	6.285	Loading	<u>}</u>
5:10	157192	100	190	10		6.45	6.16	6.305		
5:2º				10		6.46	618	6.32		
5:30				10						-
		105	225	D		7.70	X.06	X.38	Landin	29_0
5:30	199508	125	200	15		7.72	7.08	7.40		
5:45	19			15		7.73	7.10	7.415		
6:00				15		7.74	7.12	7.43		
6:15				15		7.74	7.14	7.44		
6:30										
	0/11/00/	150	280	0	0	3.90	9.57	9.135		
6:30	241,824	120		15	0	0.95	9.60	9.775		
6:421										

Signature Load Test Firm

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Signature Contractor



Ground Floor, House – 14, Road-9, Block – C, Mirpur – 12, Dhaka – 1216 Cell: 01758-835438, 01521-221012, Email: bdsoil.foundation.a gmail.com, Web: bdsoilfoundation.com



3	Sheet of	ata Recod S	Field D	J DATA KI	EST FIELI	OAD T	L PILE L	ATIC AXIA	ST			
		Average	e Reading	Dial Gage Reading								
	Remarks	Settlement	M2	M1	Flansed	Time	Draccura	Percent Of	Load on	Date		
		(mm)	(mm)	(mm)	Time (min)	(min)	Pressure 2	Design	Test Pile	ěž Tr:		
	Col-10	Col-9	Col-8	Col-7	Col-6	Col 5	(kg/cm <sup>-</sup> )	Load (%)	(Kg)	Time		
		9.815	9.64	9.99	COPU	115	0.01-4	150	Col-2	C0-1		
		9.87	9.70	10.04		15	200	120	29184	X .00		
		9.895	9.72	10.07		15				X 115		
		7.1				15				7:30		
		11.945	11.83	12.06		6	000	175	07044	7100		
		12.00	11.90	12.10		15	1.3.2.0	145	219938	<u>×. 30</u>		
		12.035	11.93	10.14		15				1.45		
		12.065	11.96	12.17		10				\$100		
		12.08	11.98	12.18		10				8:15		
				~		15			<u> </u>	8:30		
		17.21	17.94	16.48		0	01213	0.00		2100		
		17.285	18.02	16.55		15	362.10	200	32000	8:50		
1		17.36	18.11	16.61	A	15		-		8:45		
		17.44	18.18	16.70		15				9900		
		17,50	18,24	16.76		15	· · ·			5:15		
		17.54	18.28	16.80		(1)			/	9:30		
		17.59	18.34	16.85		60			2	10:30		
		.17.669	18.42	16.91		60				11:30		
		17.715	18.47	16.96		60				12.30		
		17.735	18.49	16.98	1	60				1:30		
1		17.75	18.50	17:00		60				2:30		
		17.75	18.50	17:00		60				13:30		
		17.75	1-8.50	17:00		CD				4:30		
		17.75	18.50	1700		60				5.30		
		17.75	18.50	17.00		60			1	6.50		
		17.75	18.50	17.00		60				1:30		
Ι.										8.50		
Hin	5Unbo	17.38	18.26	16.51		0	280	150	74109	0:20		
		17.34	18.21	16.47		15		1100	1-1104	0.00		
		17.32	18.19	16.45		15				a' 10		
		17.31	18.18	16.44		15				0.10		
		17.31	18.18	16.44		15				9'20		
1										12.30		
1						· .						

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Signature Contractor

Signature Client

Ground Floor, House 14, Road-9, Block C, Mirpur 12, Dhaka 1216 Cell: 01758-835438; 01521-221012, Email: bdsoil.foundation.g gnail.com, Web: bdsoilfoundation.com

# **BD SOIL & FOUNDATION Geotechnical Engineering & Consultant Services**

# STATIC AXIAL PILE LOAD TEST FIELD DATA RECORD SHEET

STATIC AXIAL PILE LOAD TEST FIELD DATA RECORD SHEET Field Data Recod Sheet of L										
						Dial Ca	ne Reading	Average		
Date	Load on	Percent Of	Observed	Elapsed	Cumulative	M1	M2	Settlement	Remarks	
&	Test Pile	Design	Pressure	Time	Etapseu T'un (min)	(mm)	(mm)	(mm)		{
Time	(Kg)	Load (%)	(kg/cm <sup>2</sup> )	(min)	Time (min)	Col-7	Col-8	Col-9	Col-10	1
Col-1	Col-2	Col-3	Col-4	Col-5	COF0	10 57	14.67	13.62	Valloo	mg-
9:30	157192	100	190	0		10 50	111.59	13,54	10,000	
9:45				15		12.50	14.59	13,54	1	
10:00				15		12.50	111.59	13.54		
10:15				15		12.00	III.D	13.54		
10:20				15		12.50	19.2			
10,00						0.50	0 94	8,915	Vnlood	mg
10:30	8944	50	110	0		8.59	0.01	8,895		
INING	4/01	~~~		15		8.55	9.24	8.89		
10.00				15		8.54	9.29	8.865		
11/16				15		8.52	5.2	0.865		
11.20				15		8.52	9,21	0.0.0		
11,30					1	Sec. 1	-00	5 175	Umbod	ng
11100	157	070	00	0	de la	4.95	5.20	5 005		V
11.30	00			15	South Sec.	4.87	5.19	11.96		
11:45				15	1 80	4.81	5.11	1.955		
12.00				15	1.2.1	4.80	5.11	1.900	FND	
12:15				15		4.80	5.11	4.000		
12:30				1						
	-		10~							
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Load Test Firm

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## **PHOTOGRAPHS**

